U.S. ENGINEERS-LAKE LONGYS

A meeting was held in Elocmington, Indiana, on Friday, September 15, 1961, to review the road system that will be needed for access to Monroe Reservoir. The county surveyors and county commissioners of Brown, Jackson, Lawrence, and Monroe Counties attended the meeting, during which they outlined the routes they agree will be absolutely essential in their respective counties in order to tie into the existing state and federal highway systems as well as to promote and maintain reasonable routes for local traffic.

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By using as a guide the maps prepared by the Indiana Flood Control and an para Para anno ann an an air an agus reagailte an Raigh ghairean an gun air bheag an bhiad ann a Water Resources Commission entitled "Monroe Reservoir, Public Lands and Federal Aid Roads" (dated November 1960) and "Monroe Reservoir Access Roads" The same of the same of the same of the same (dated August 1960) and the appropriate topographic sheets prepared by the and the state of U. S. Geological Survey, the group outlined the following routes as those they The state of the s believe absolutely essential to best serve the area, its residents, and its visitors. The routes listed will in part be needed to replace existing routes that will be disrupted by construction of Monroe Reservoir. They have been extended to a point where they tie into existing State or U. S. Highways. The entire route of each will need widening and major reconstruction, however, if it is to carry the volume of traffic the group anticipates will move through the Monroe Reservoir area.

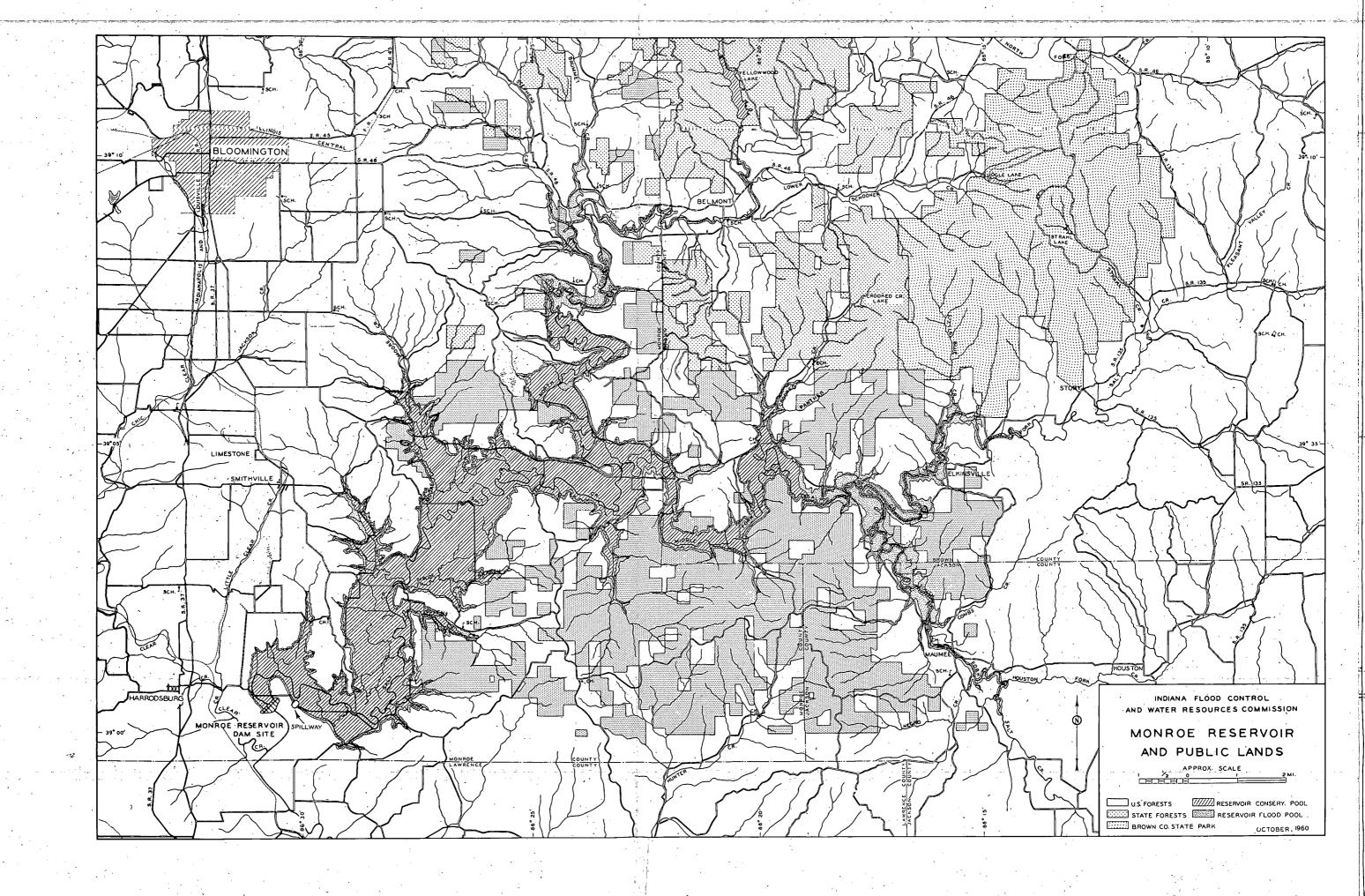
A route will be essential to move traffic, both tourist and local, through the central part of the lake area from both north and south without creating any additional congestion on Indiana Highway 37 and the roads that lead eastward from it into the reservoir area. To move this traffic most conveniently, Road S-670 south of Indiana Highway 16 should connect with Road S-988 to go across a causeway (shown as "proposed" on some maps) near the center of the reservoir. The route should continue southward, from the causeway about 3 miles, thence southwestward on the road through Chapel Hill,

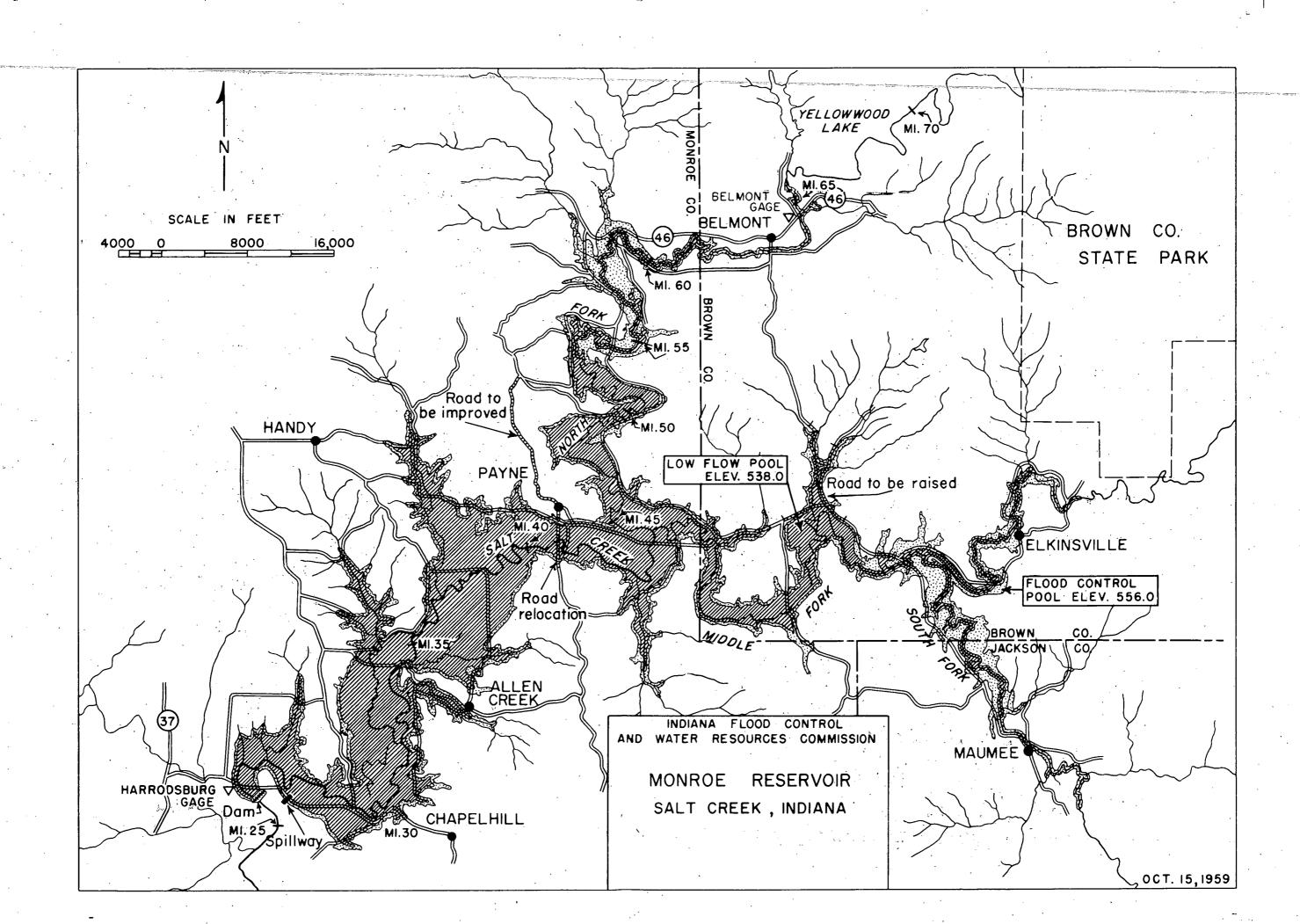
thence continuing into Road S-983 in Lawrence County via either the road southwest from Chapel Hill, by Road S-202 to Bartletteville, or both.

A good route going southeastward from the reservoir must be built and maintained in order to adequately handle traffic headed to the lake that arrives in Seymour and Brownstown via U. S. Highway 31 and Interstate 65 from Louisville and other cities southeast of the lake. The route for this road that is favored by and most strongly recommended by the commissioners of the counties involved is one that would follow or closely parallel Road S-976 through Maumee northwestward to a proposed causeway near the eastern end of the lake. From this causeway the route would follow Road S-671 northward to Belmont.

A further justification offered for the need for this route by local residents is the considerable commuting that takes place between Jackson County and Indiana University. Without a direct road northwestward, these persons would find it necessary to follow a much longer detour to reach their places of work and study.

The commissioners and surveyors of all counties represented in this meeting pointed out that they will be unable within their counties to raise sufficient funds either to build these needed roads or to meet Federal Aid matching funds. Construction, therefore, will have to be done by as part of the reservoir project or by state or federal highway authorities.





INTER-DEPT. MEMORA MONROE COUNTY HIGHWAY

Subject: \_\_\_\_\_

17 TO 17' - N36-4614 233.4 19' 70 18' - N.36- 46W 34.5

18 70 18' - NZO - 31 E = 137.7

181 TO 19- N 20- 31E= 4.7 15 TO 19'- N47-51 W- 181.8 = 909

19' TO 20 = N47- 51 W - 126 20-70 Zo! =14 44-05F-141.5 20-70 Z1 = N44-USE- 83.5

27 TO 23- = N39-16E- 215.5 23 TO 24 = 5-88-04-W- 256.0

21 To 21 12 N-34-53 W-21'70 22 N34.53 W 84.5

24 TOZS = 5-14-39-W-174,5 25 7026: = 5-86-50W- 1365

\$26 TO Z6' = N-40-27W- 110.00 26' 70 27 = N-40.27-W- 174.4

27 TO 28- - N-07- 51E- 119,3 28 729 = N-33-21 E. 174.3 16517 - N-4-15W- 31414

157616= N-14-52 E,-572.8

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MONROE RESEROIR
MARS

James a Lewis .
Box 5999
Louisvilla My.
Chang Engineers

OHIO RIVER BASIN

# MONROE RESERVOIR, INDIANA

RELOCATION OF ATE AND COUNTY ROADS \$ ES 1, 3, 4



U.S. ARMY ENGINEER DISTRICT, LOUISVILLE
CORPS OF ENGINEERS
LOUISVILLE, KENTUCKY

ALL DRAWINGS IN THIS FOLIO HAVE BEEN REDUCED ONE HALF THE ORIGINAL SIZE

## MONROE RESERVOIR, INDIANA

RELOCATION OF STATE AND COUNTY ROADS
\*SITES 1, 3 AND 4

EFW 143-12.4/I INDEX SHEET EFW 143-12.4/2 GENERAL PLAN

#### SITE I RELOCATION OF IND. ROUTE 46

EFW 143-12.4/3 PLAN & PROFILE - SHEET I

EFW 143-12.4/4 CROSS SECTIONS - SHEET I

EFW 143-12.4/5 CROSS SECTIONS - SHEET 2

EFW 143-12.4/6 CROSS SECTIONS - SHEET 3

EFW 143-12.4/7 CROSS SECTIONS - SHEET 4

## SITE 3-RELOCATION OF MONROE CO. RD. - AT RAMP CREEK

EFW 143-12.4/8 PLAN & PROFILE - SHEET I

EFW 143-12.4/9 CROSS SECTIONS - SHEET I

EFW 143-12.4/10 CROSS SECTIONS - SHEET 2

EFW 143-12.4/11 CROSS SECTIONS - SHEET 3 EFW 143-12.4/12 CROSS SECTIONS - SHEET 4

EFW 143-12.4/13 CROSS SECTIONS - SHEET 5

EFW 143-12.4/14 CROSS SECTIONS - SHEET 6

EFW 143-12.4/15 CROSS SECTIONS - SHEET 7

## SITE 4- RELOCATION OF MONROE CO. RD. - AT SUGAR CREEK

EFW 143-12.4/16 PLAN & PROFILE - SHEET !

EFW 143-12.4/17 CROSS SECTIONS - SHEET 1 EFW 143-12.4/18 CROSS SECTIONS - SHEET 2

EFW 143-12.4/19 CROSS SECTIONS - SHEET 3

EFW 143-12.4/20 CROSS SECTIONS - SHEET 4

### SOIL INVESTIGATION - SITES 1, 3 AND 4

EFW 143-12.4/21 SOIL CLASSIFICATION - SHEET I EFW 143-12.4/22 BORING LOGS & PROBINGS - SHEET 2

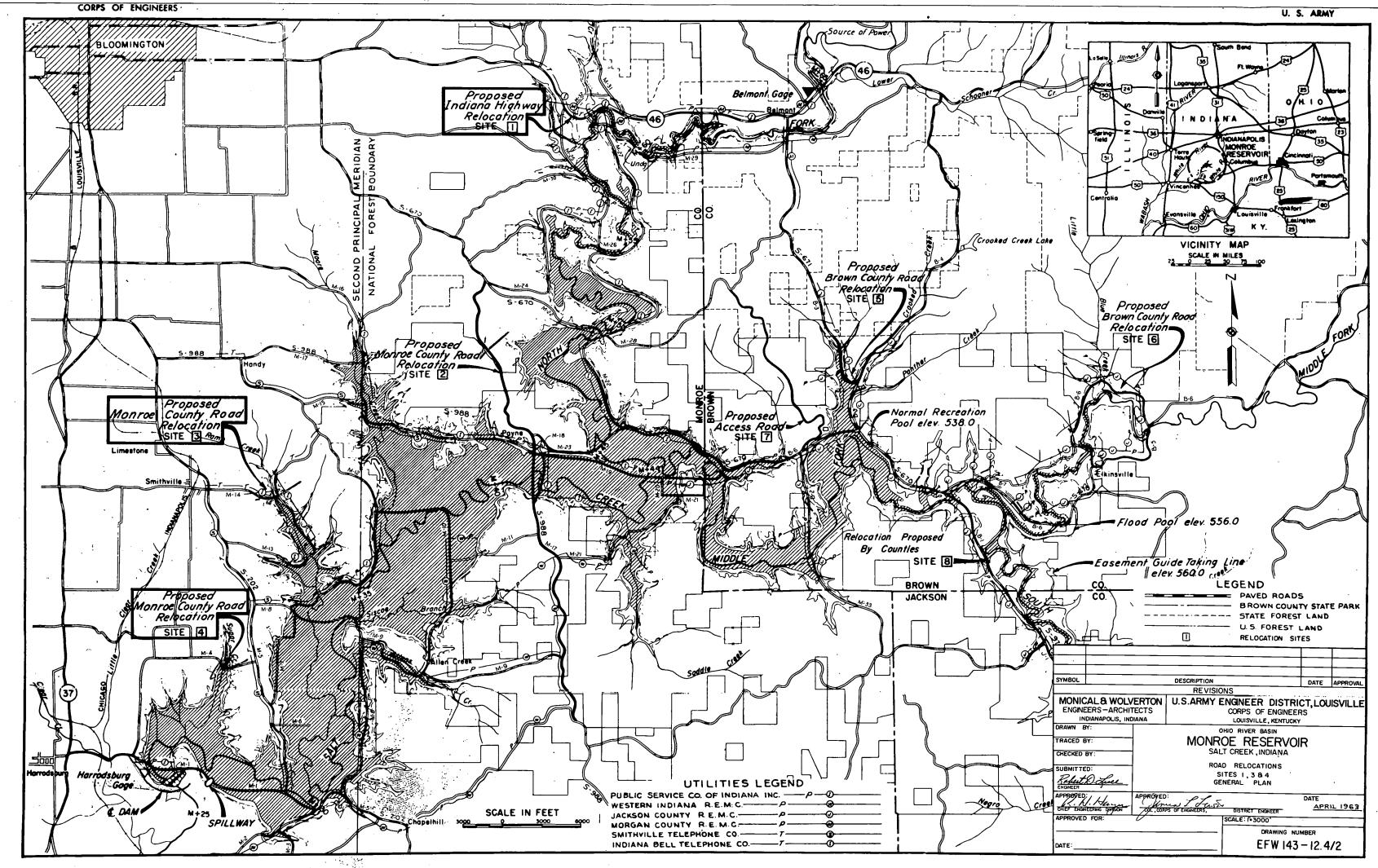
#### ROADWAY DETAILS

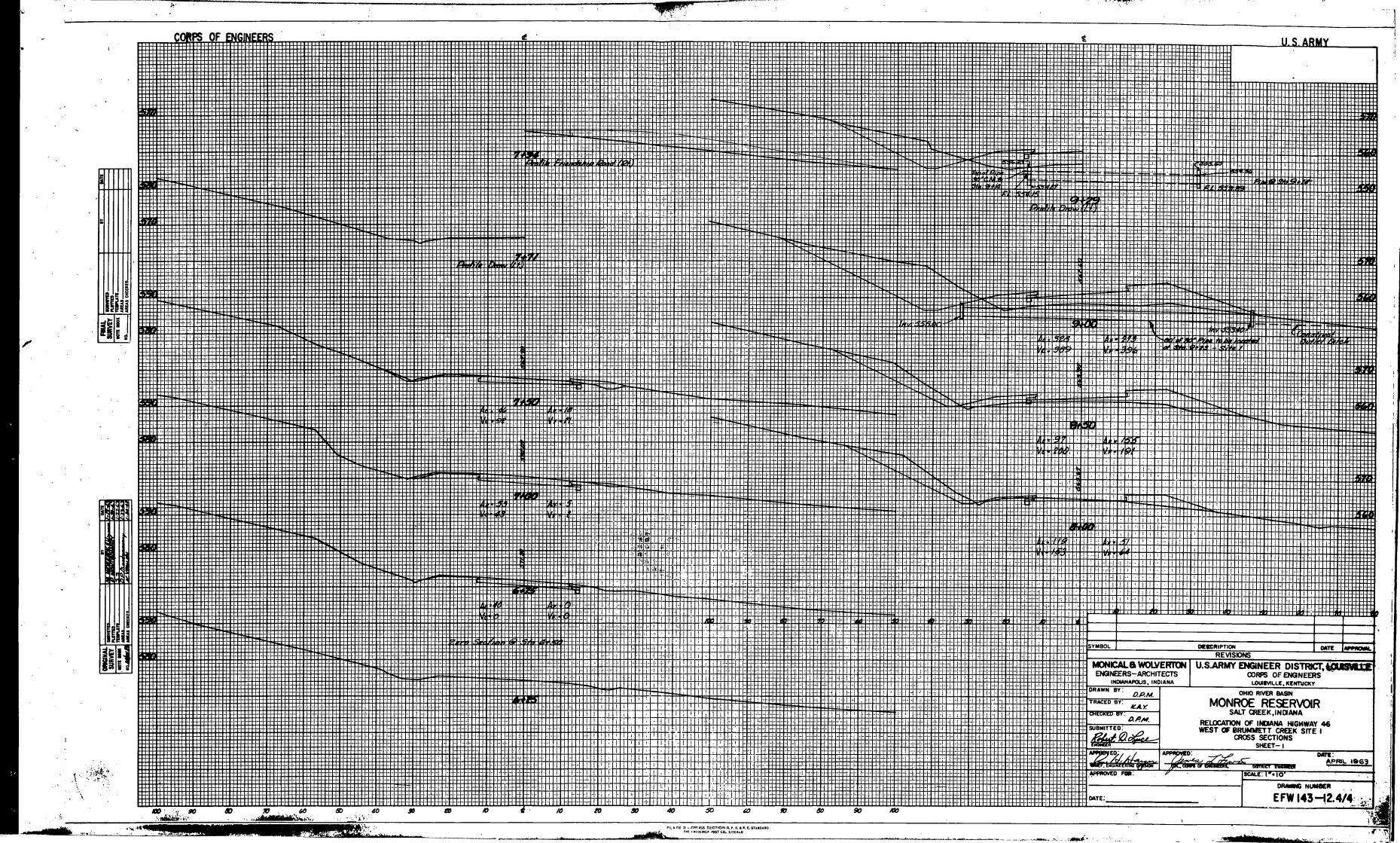
EFW 143-12.4/23 MISCELLANEOUS DETAILS - SHEET I

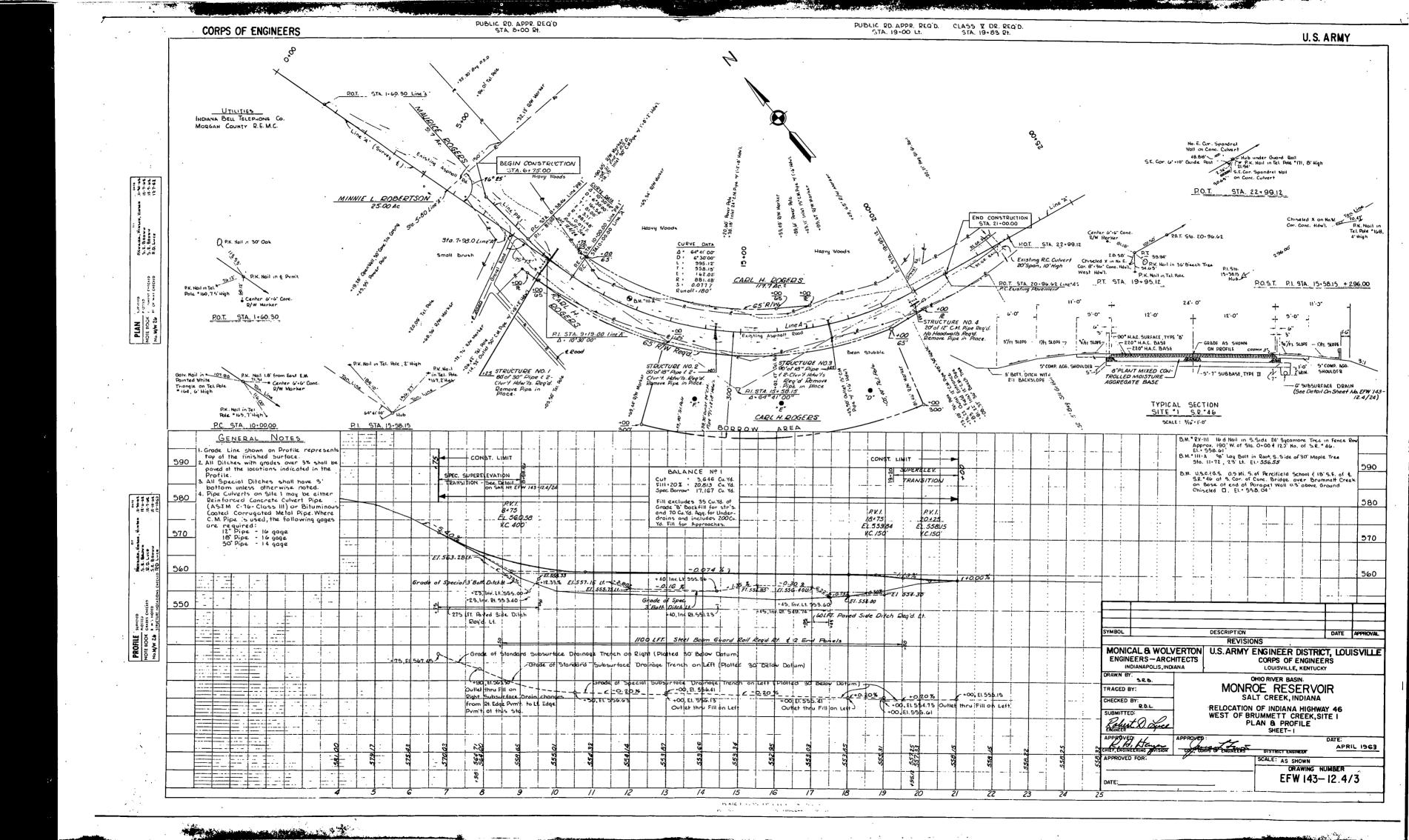
EFW 143-124/24 MISCELLANEOUS DETAILS - SHEET 2

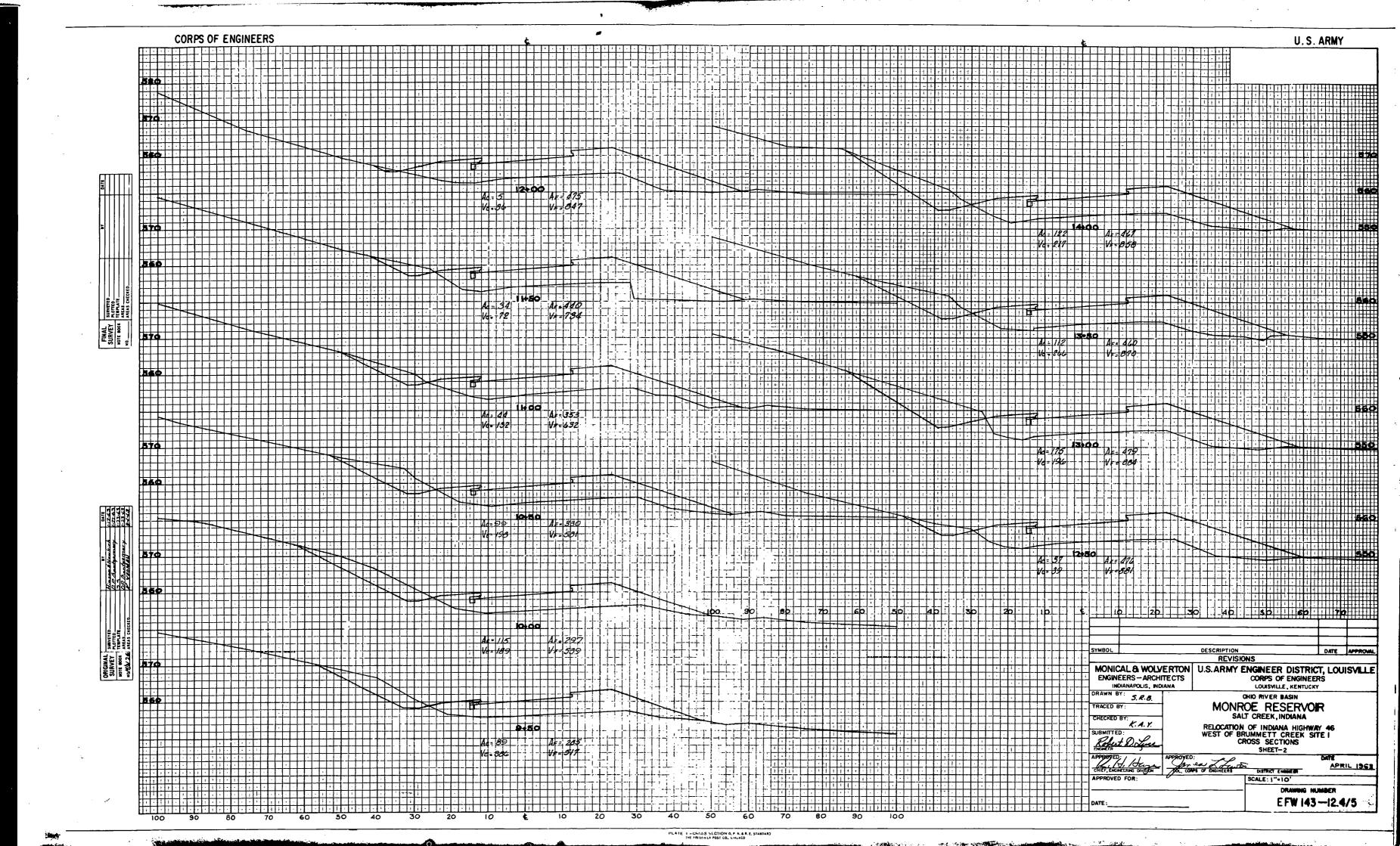
EFW 143-12.4/25 BOX CULVERT DETAILS - SHEET 3

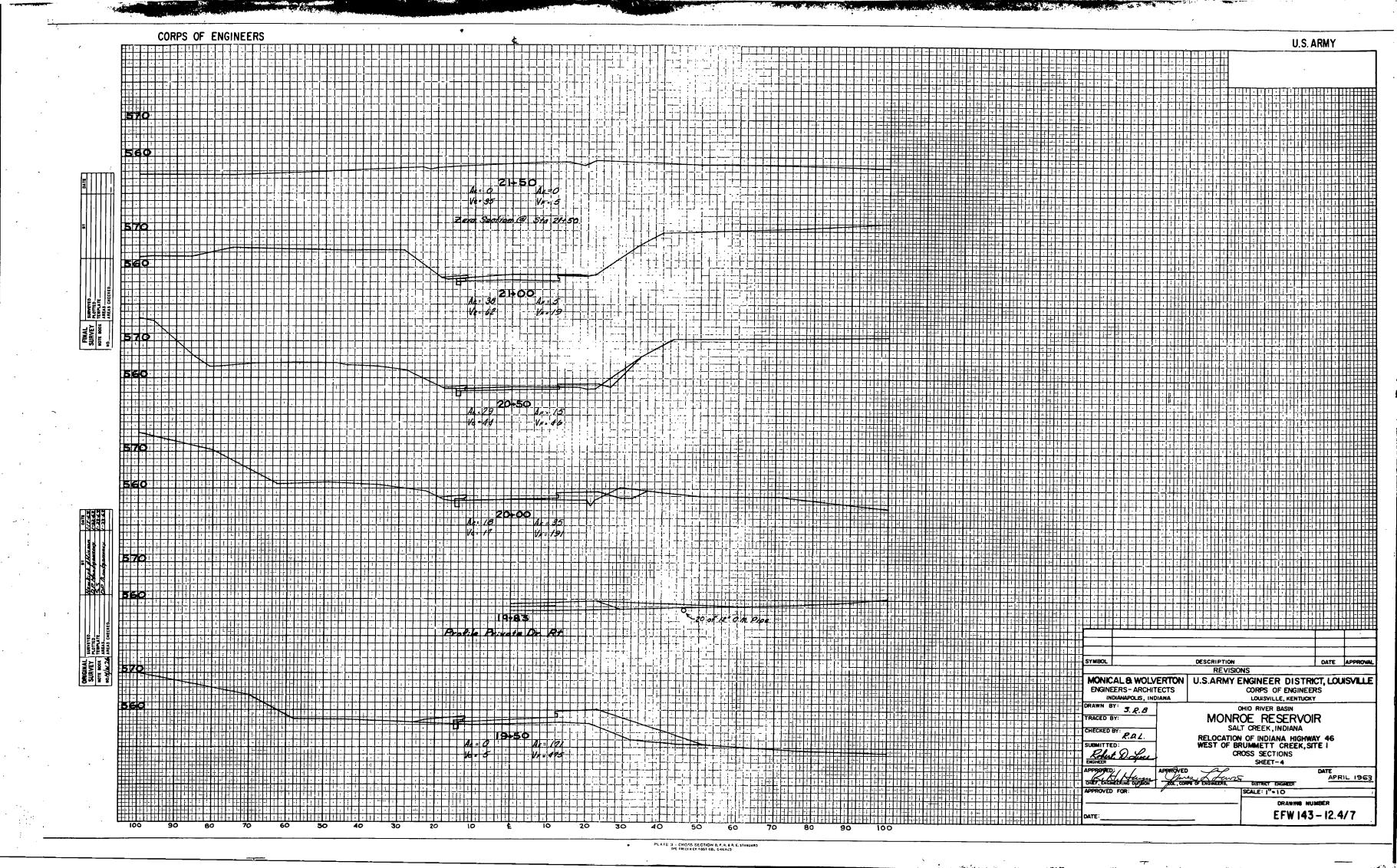
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MONICAL & WOLVE ENGINEERS - ARCHITE INDIAHAPOLIS, INDIA	CTS	U.S.ARMY	CORPS	ER D OF EN	GINEER	T, LOU	JISVIL
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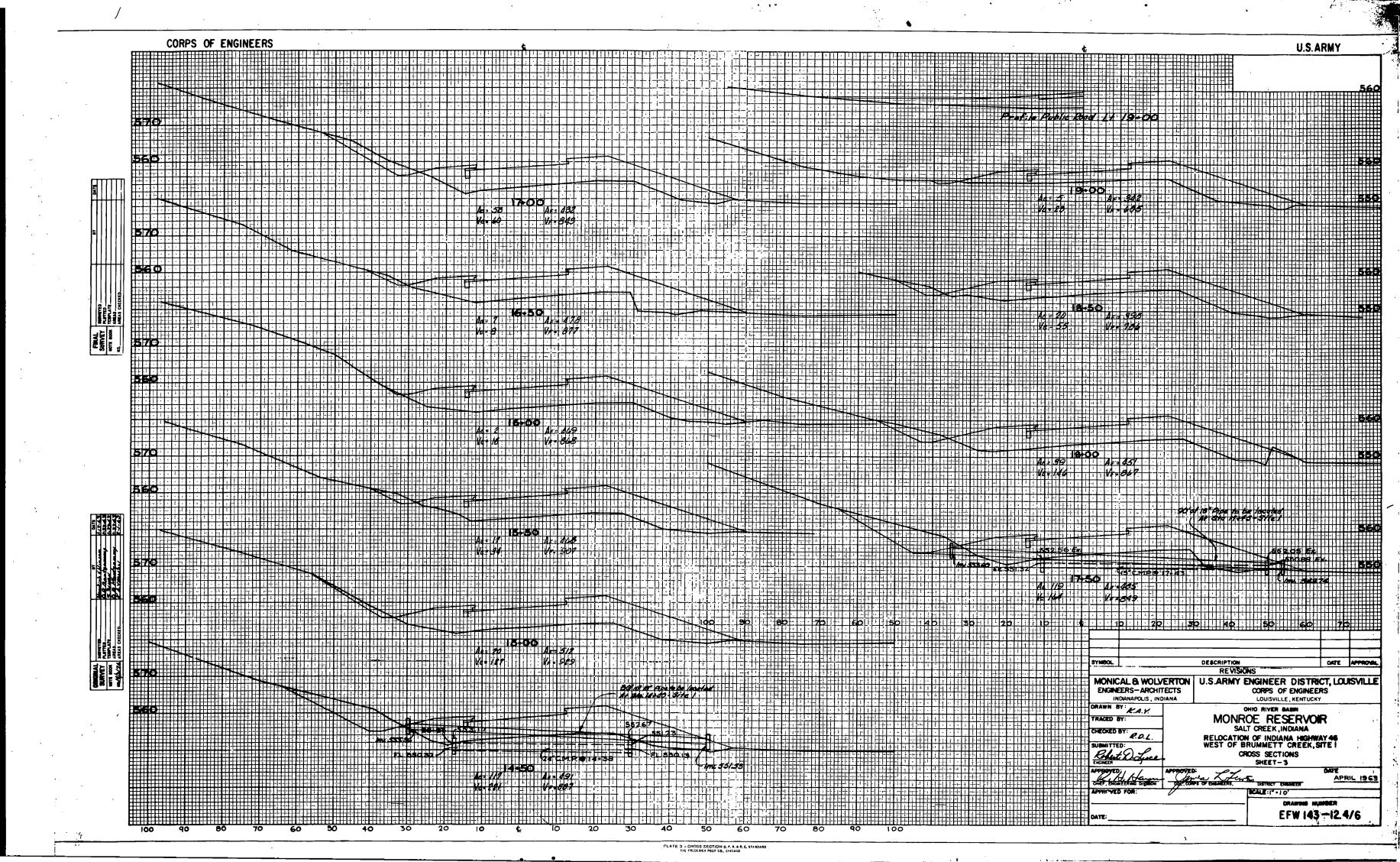


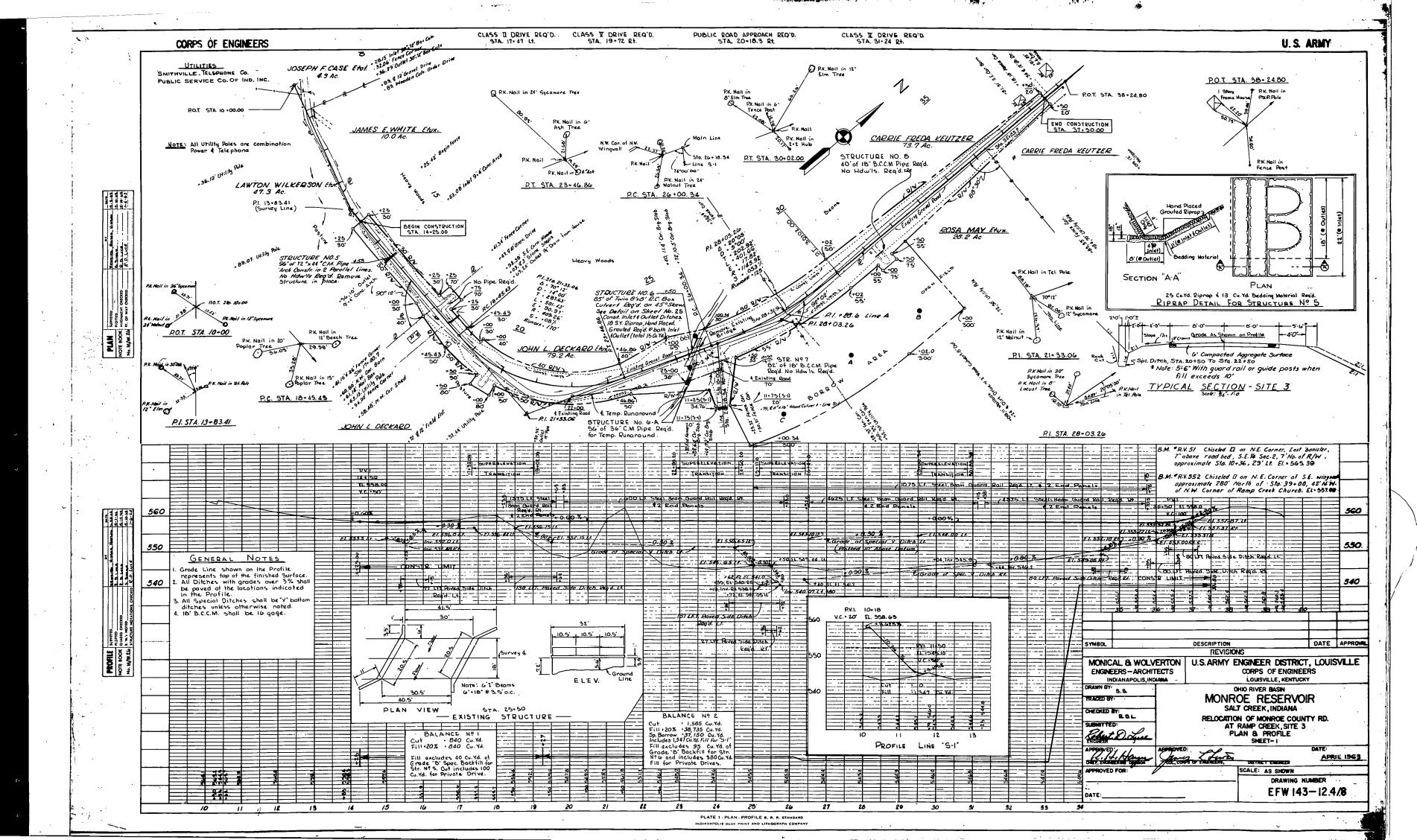


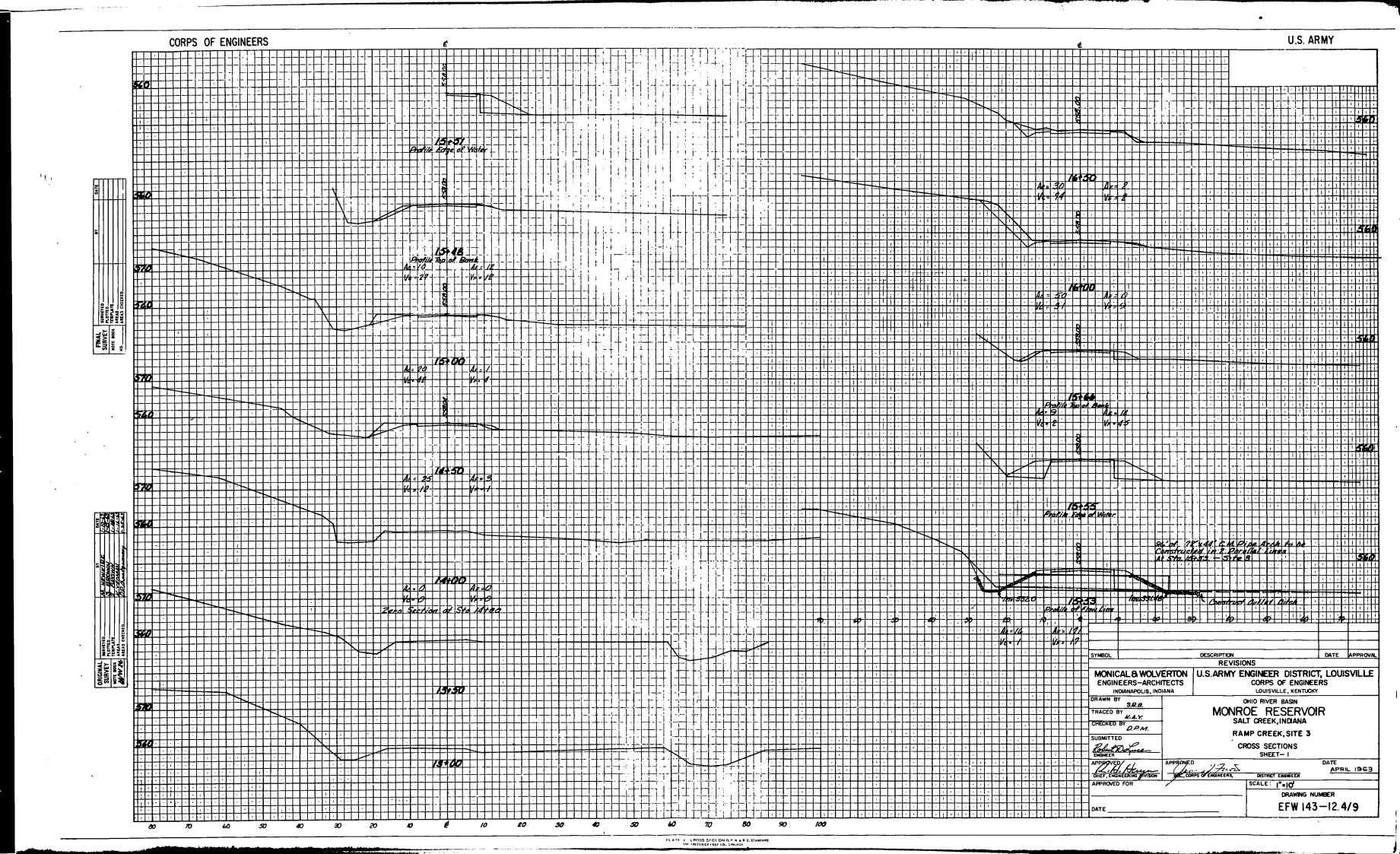


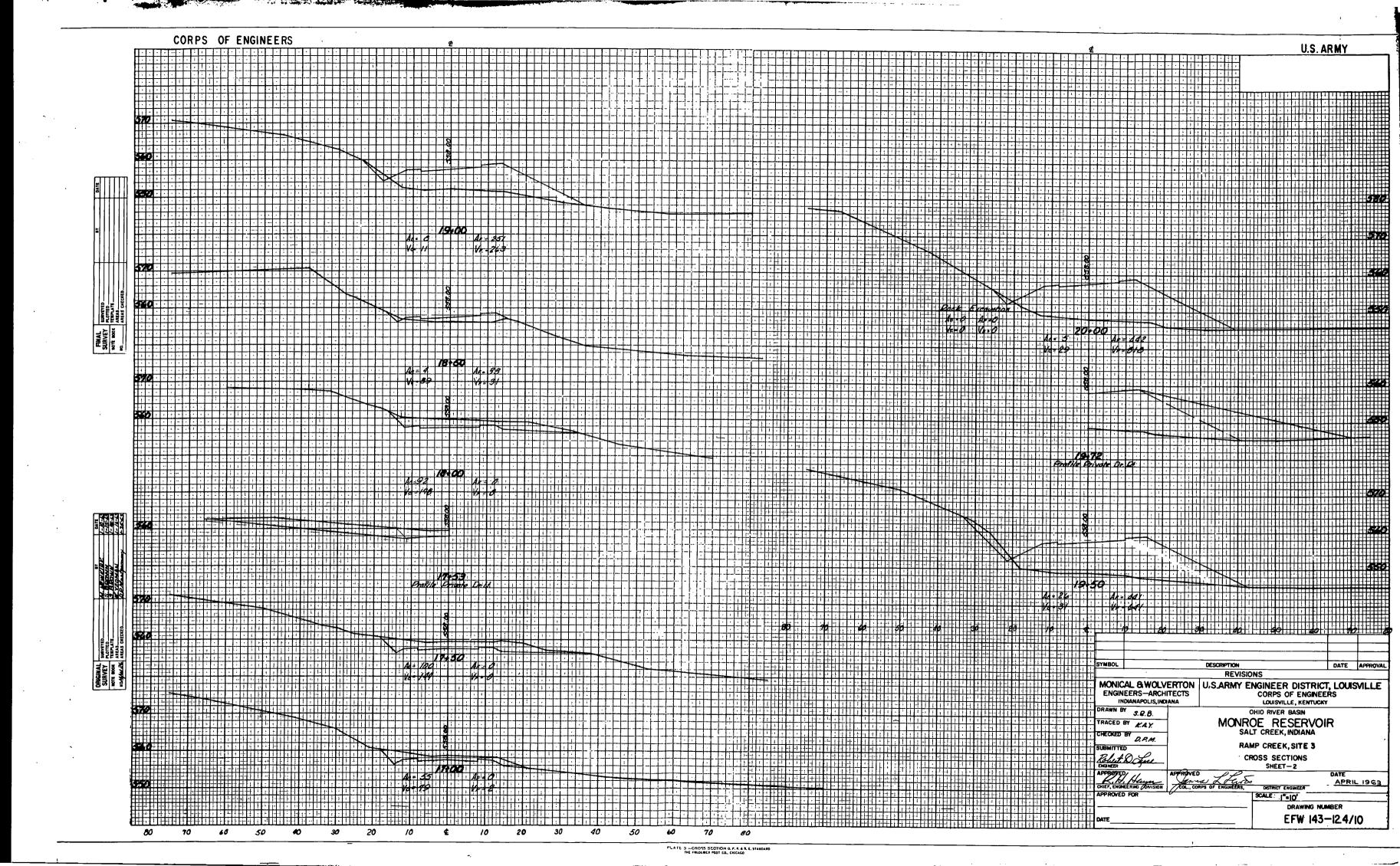


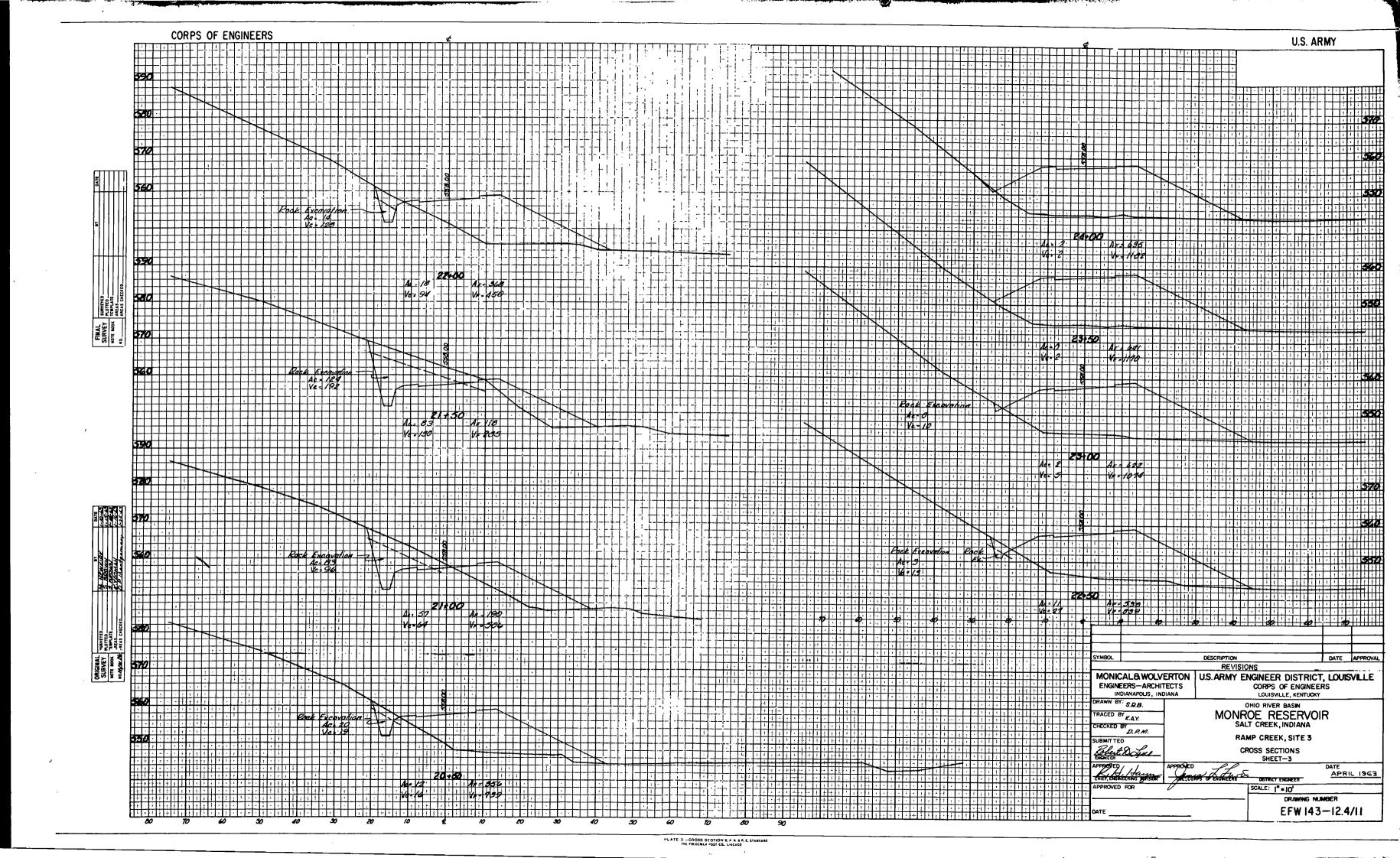


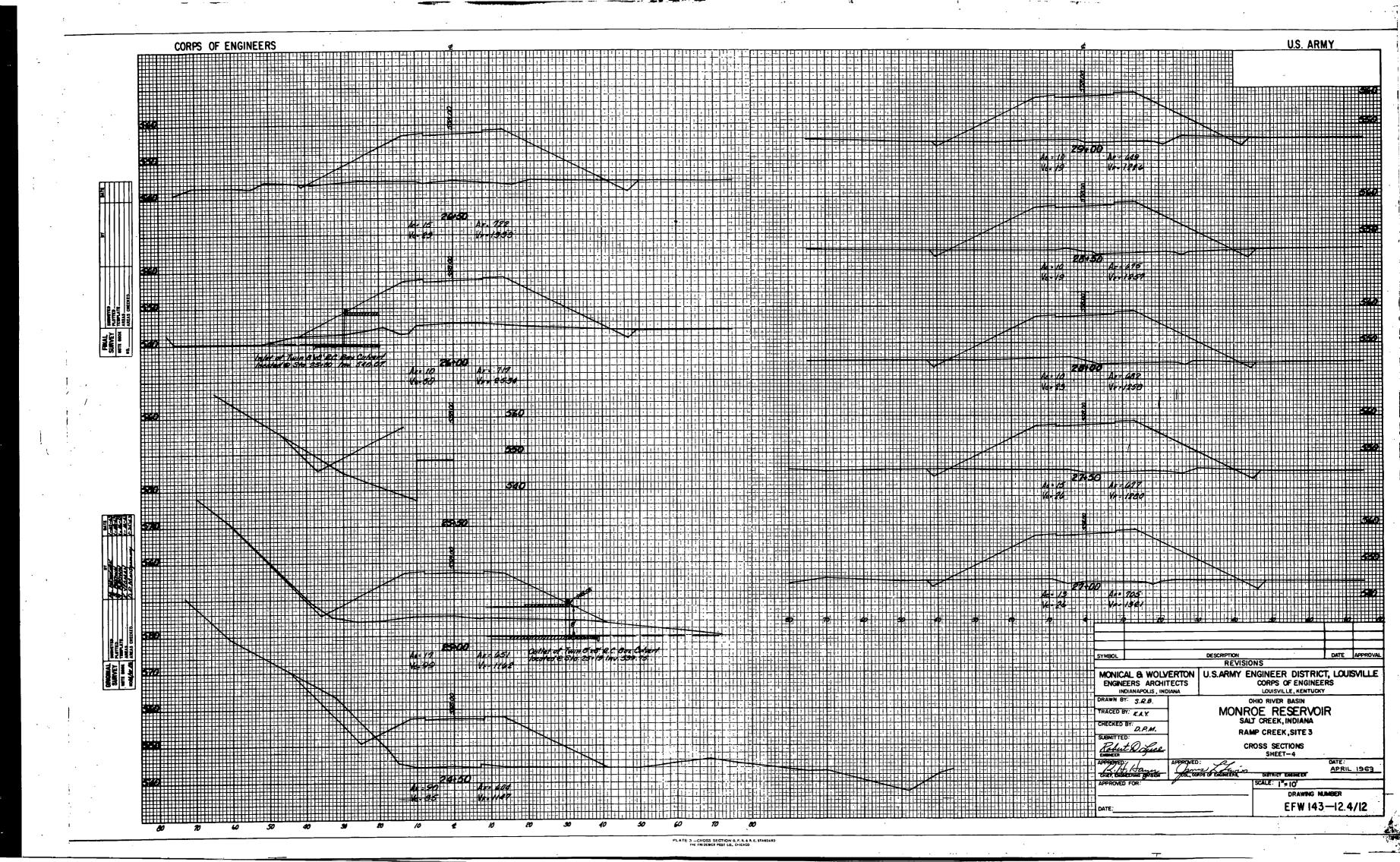


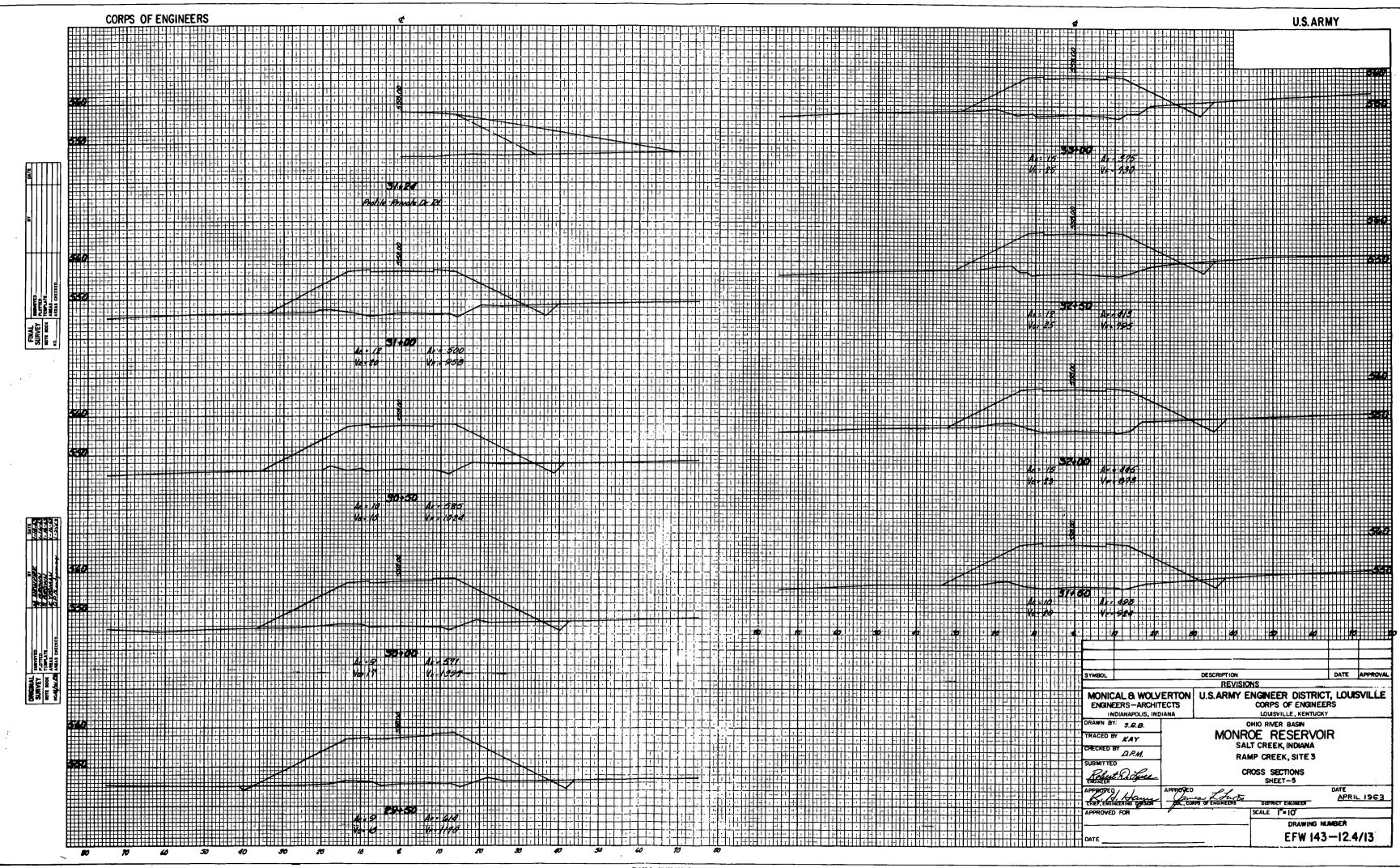


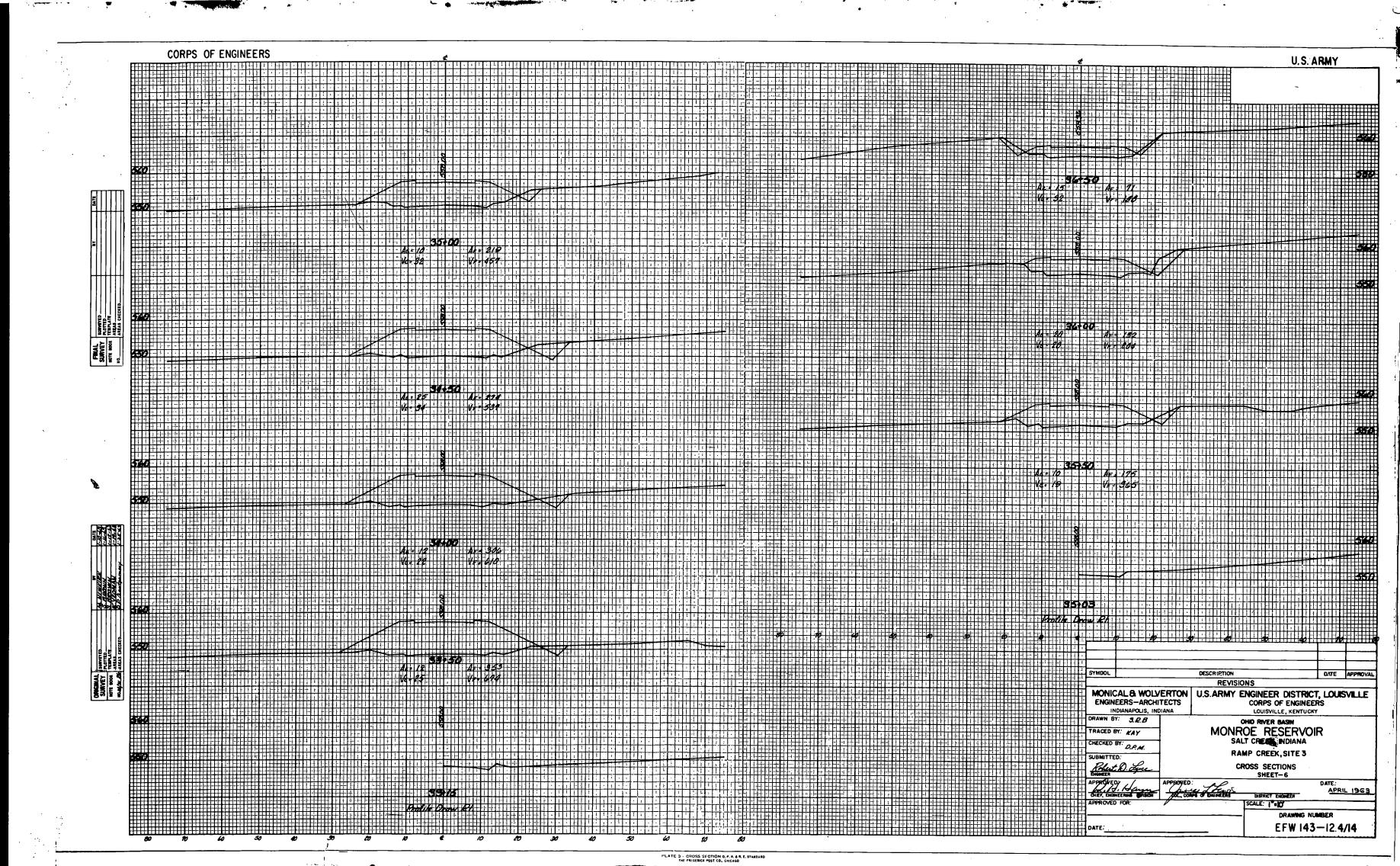


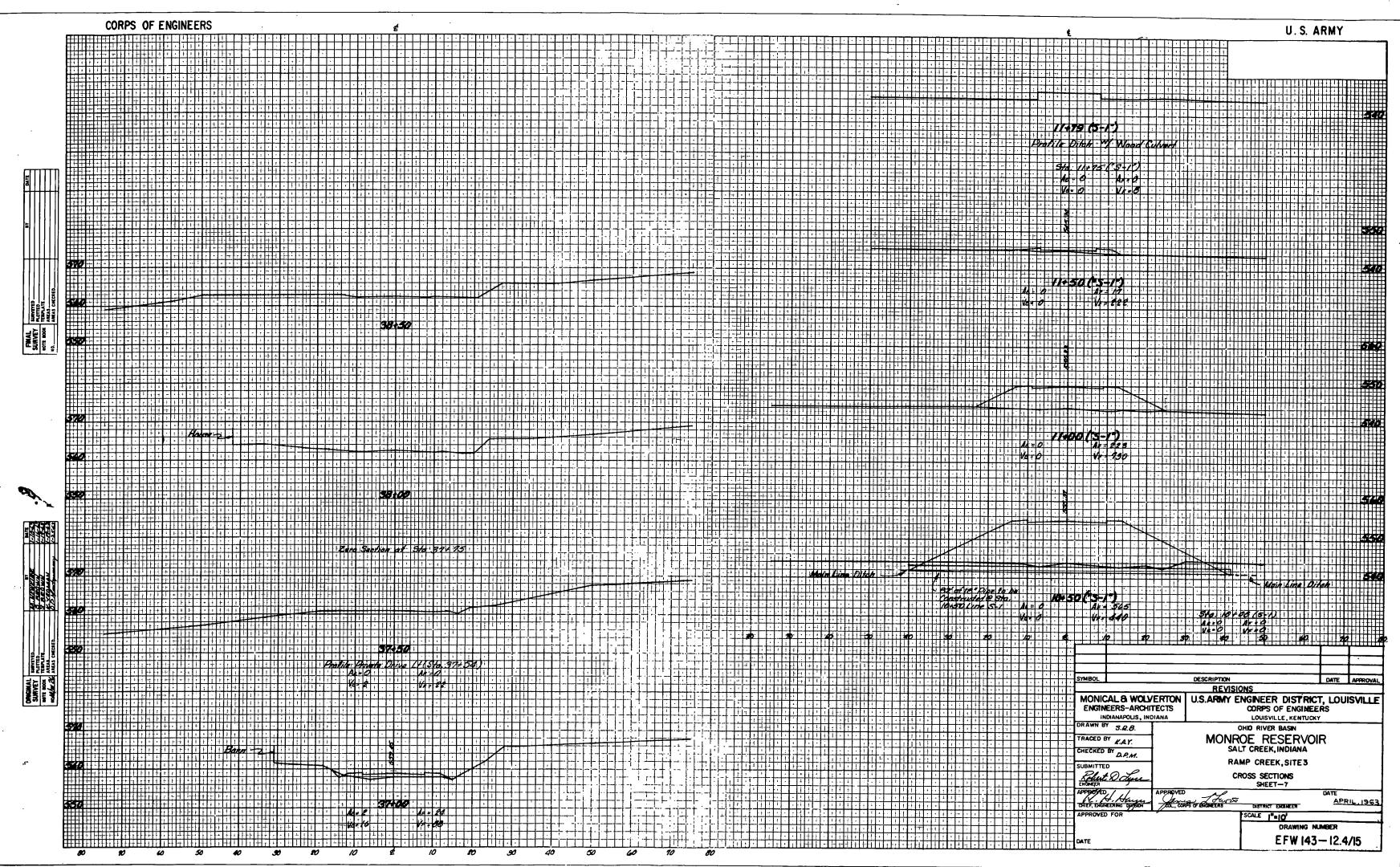


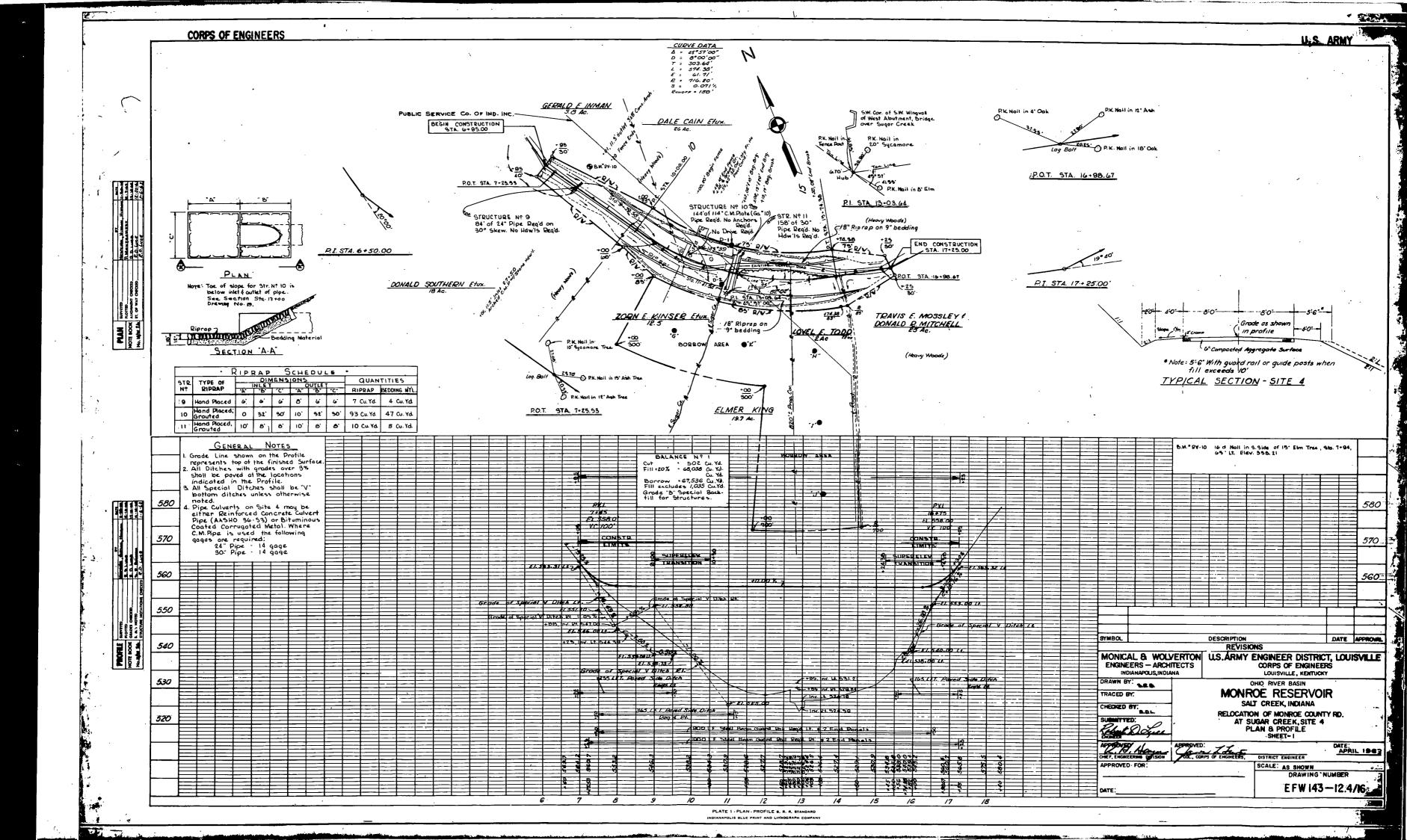


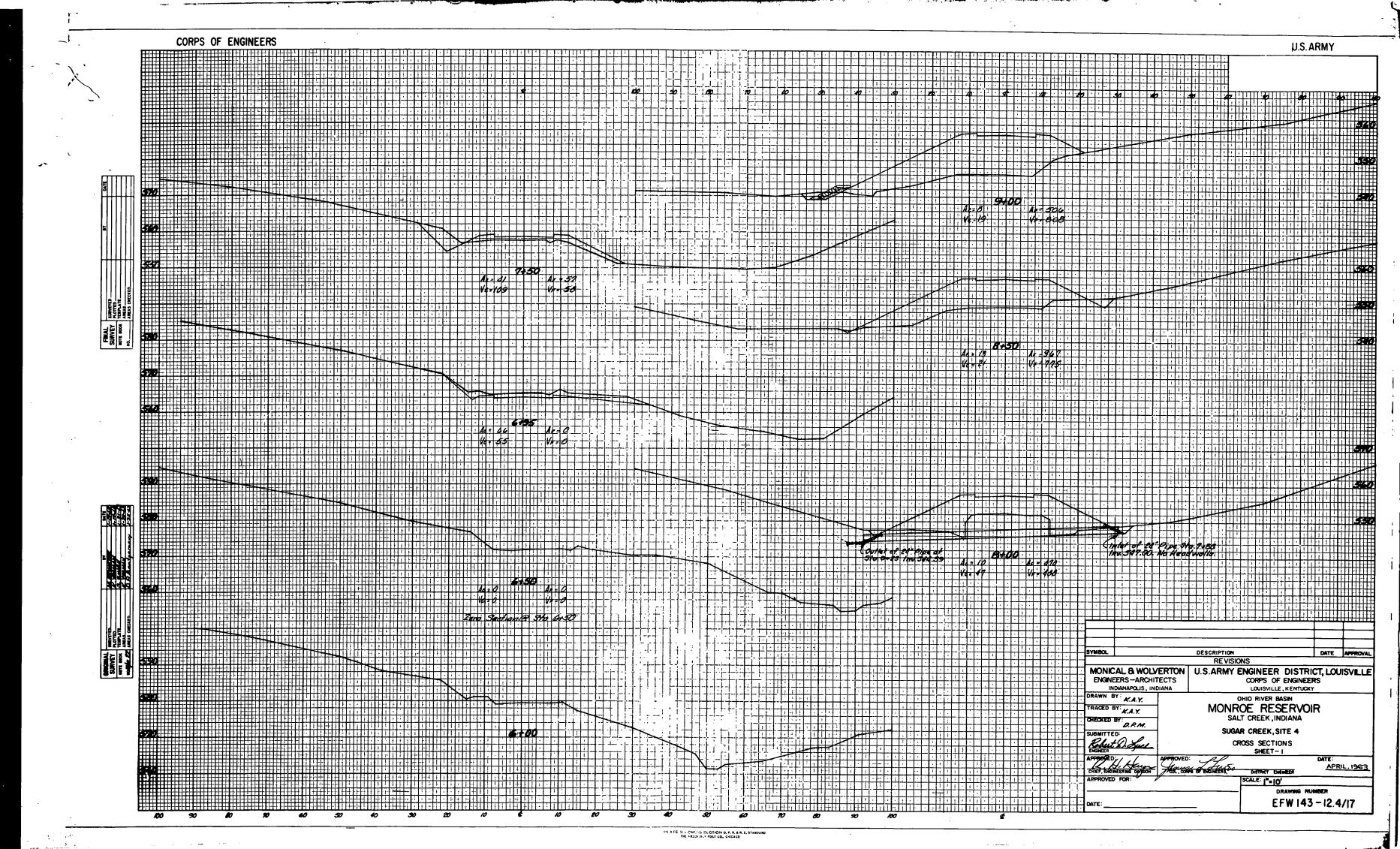


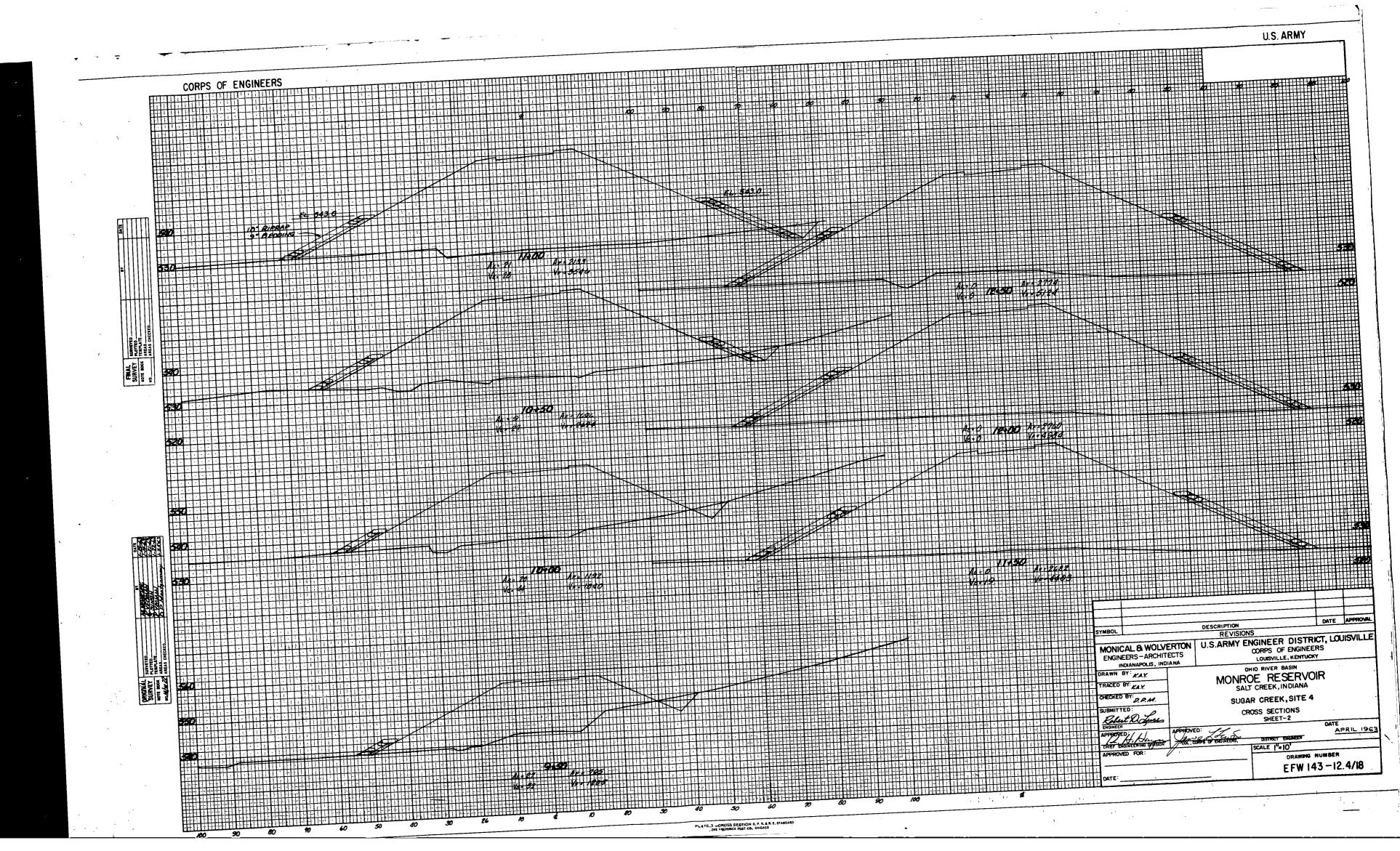


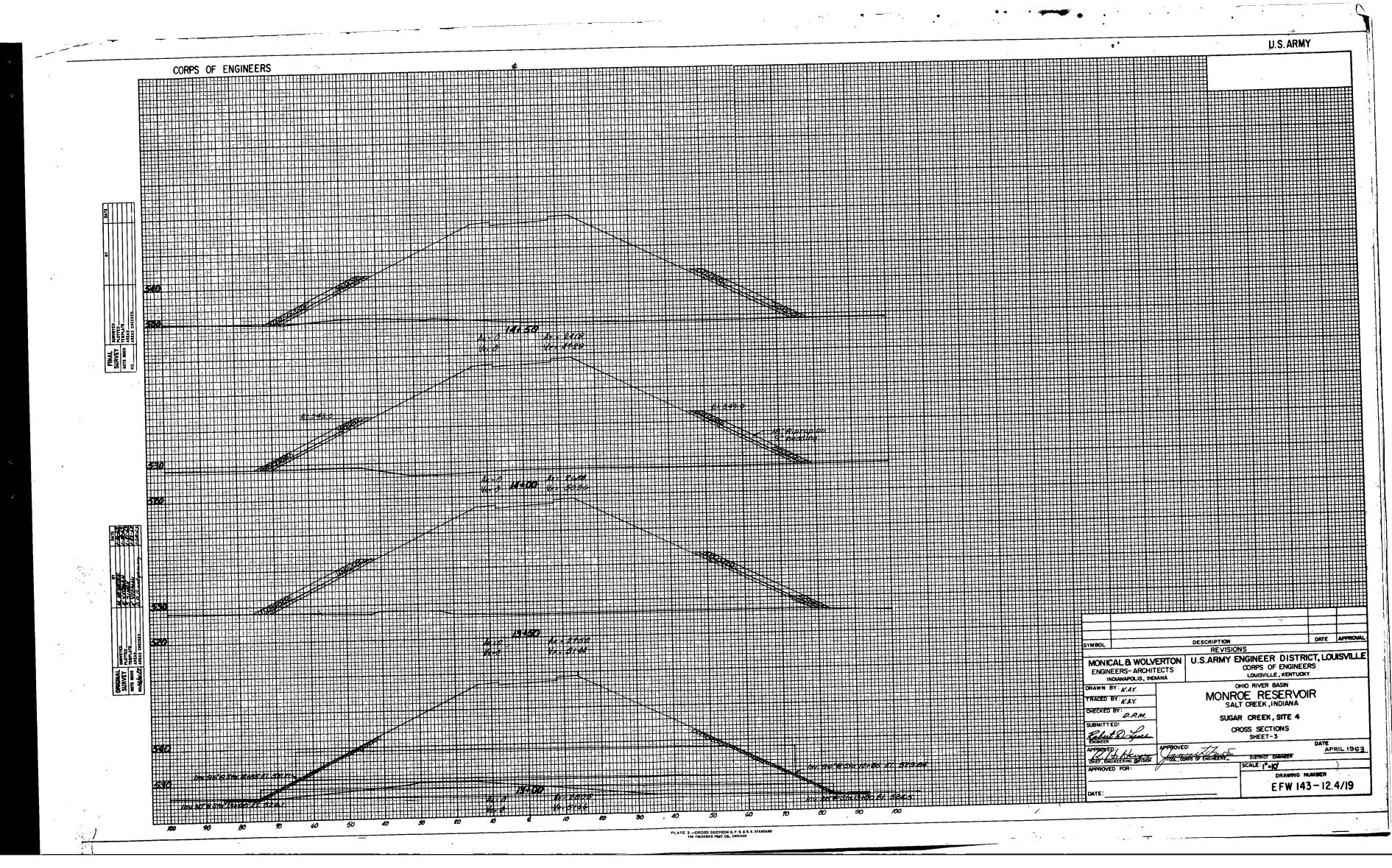


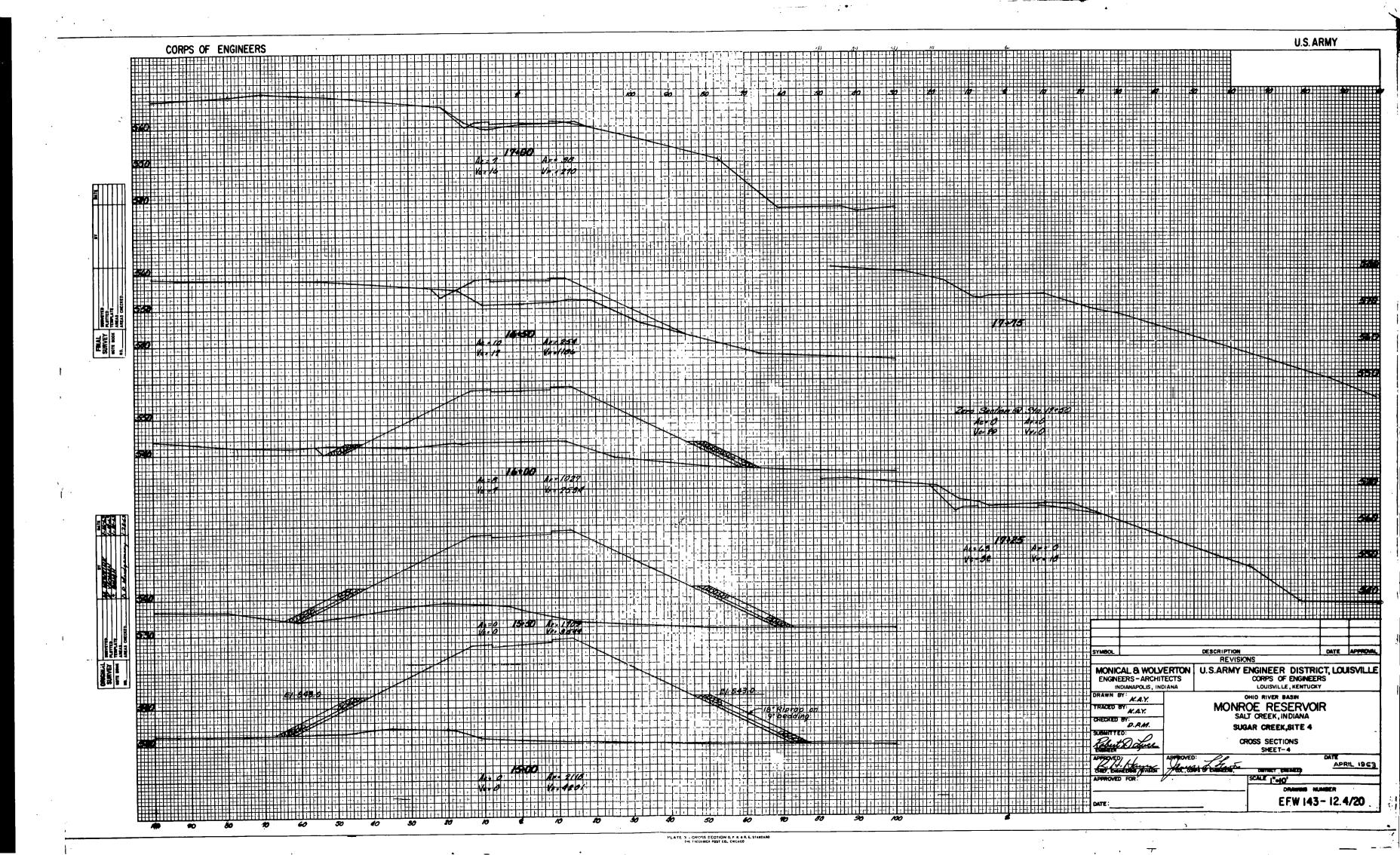












#### CORPS OF ENGINEERS

_						(Including Id	IED SOIL CLA					
_					Field Id	tentification Proc	egures	Information Required for			Laboratory Classification Criteria	
Major Divisions S		Group Symbols	Typical Names	(Excluding particles larger than 3 inches and basing fractions on estimated weights)		Describing Soils	7					
		2	3	4	5			6			$C_{\mu} = \frac{D_{60}}{D_{60}}$ Greater than 6	
rained Soils is <u>larger</u> than No the naked eye.	fraction size. uivalent	<u> </u>	GW	Well - graded gravels , gravel - sand mixtures, little or no fines.	Wide range in grain sizes and substantial amounts of all intermediate partical sizes.		For undisturbed soils add information on stratification, degree of compact ness, cementation, moisture conditions		o grain-size curve.  er than No.200 s follows: , SP. , SC. , SC.	C <sub>C</sub> : D <sub>1O</sub> Steller Man C (03 <sub>O</sub> ) <sup>2</sup> Detween one and 3		
	Gravels  More than half of coo is larger than No.4 sie In size may be used or Gravels with Gravels with Cle Chapreciable (Lift amount)	8₹	GP	Poorly - graded gravels , gravel-sand mixtures , little or no fines.	Predominately one some intermed	e size or a ran diate sizes missi	ge of sizes with ing.	and drainage characteristics	dentification	er than s follows 1, SP . A, SC . cases r	Not meeting all gradation require  Atterbery limits below "A" line	ments for GW Above "A" line with
			GM	Silty gravels , gravel - sand - silt mixtures.	Nonplastic fines ( for identification	or fines with lo on procedures se	w plasticity e ML below ).	Give typical name, indicate approximate		es (fraction smaller than es (fraction smaller than are dassified as follow GW, GP, SW, SP. GM, GC, SM, SC. Border line cases	or PI less than 4  Atterberg limits above "A" line	PI between 4 and 7 are borderline cases requiring use of dual
		sleve sta sravets w Fine: Apprecta amoun	GC	Clayey gravels, gravel-sand-clay mixtures.	Plastic fines (for see CL telov		rocedures	percentages of sand and gravel, max- imum size, angularity, surface condition, and hardness of the coarse grains;		ses (1)	<u> </u>	symbols.
		on the contract of the contrac	sw	Welt - graded sands, gravelly sands, little or no fines.	Wide range in o	grain size and sul ediate partical :	bstantial amounts sizes.	tocal or geologic name and other pertinent descriptive information; and symbol in parentneses.	under field i	Determine percentages of gra Depending on percentage of fit sieve site) coarse-grained so Less than 5% More than 12% 5% to 12%	$C_{u} = \frac{\frac{060}{010}}{\frac{010}{010}}$ Greater than 4 $\frac{(0.30)^2}{C_{c}} = \frac{0.30}{010}$ Between one and 3	
		SP	Poorly - graded sands, gravelly sands, little or no fines.	Predominately on with some inter	e size or a rang rmediate sizes m	e of sizes issing.	Example: Sitty sand, gravelly, about 20% hard, angular gravel particles 1/2 in. maximum size, rounded and subangular sand grains coarse to fine, about 15% nonplastic fines with low dry strength,		ercentag opercent oarse -g 5% 12%	Not meeting all gradation requirements for SW  Atterberg limits below "A" line Limits plotting in		
	Sonda particle visible to a fine to the smollest particle visible to the same tradition to the No. Sonda with Clean Sonds a fines to the same tradition to the No. Sonds with Clean Sonds the same transfer of the same traditions of the same traditions and the same transfer traditions and the same traditions and the same traditions and the same traditions and traditions are same traditions and traditions and traditions are same traditions are same traditions and traditions are same tr		SM	Silty sands, sand-silt mixtures.	Nonplastic fine (for identificati	es or fines with ion procedures s			low plasticity ee ML below).	etermine percente epending on percel leve size) coarse- Less than 5% More than 12%	or PI less than 4  Atterberg limits above "A" line	hatched zone with PI between 4 and 7 are borderline cases
			sc	Clayey sands, sand-clay mixtures.	Plastic fines (for identification procedures see CL below).		well compacted and moist in place, al- luvial sand, (SM).	the fractions	See Cope	with PI greater than 7	requiring use of dual symbols.	
- ei i	8			·	on Fraction Sn  Dry Strength (Crushing Characteristics)	rification Procedu nailer than No. 4 Ollatancy (Reaction to shaking)	Toughness (Consistency near PL)		Identifying th	60 =		
greined Soils steriol is <u>smaller</u> than No.200 sieve s	Seve	Silts and Clays Liquid limit less than 50		Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.	Nane to slight	Quick to slow	None	Give typical name, indicate degree and character of plasticity, amount and maximum size of coarse grains, color	curve in	50	Comparing Soils at Equal Liquid Limit Toughness and Dry Strength Increase with Increasing Plasticity Index	
sneller				inorganic clays of low to medium plasticity gravelly clays, sandy clays, silty clays, lean clays.	Medium to high	None to very slow	Medium	in wet condition, odor if any, local or geologic name, and any other pertinate descriptive information, and symbol in parentheses.  For undisturbed soils add information	Use grain-size o	Q 40	<u> </u>	
oterio! 15				Organic silts and organic silty clays of low plasticity.	Slight to medium	Slow	Slight			PLASTICITY 00	P.W.	<u>*</u>
More than half of the mat	1	<b>s</b> 00		Inorganic clays of high plasticity, fat clays	High to very high	None	High	on structure, stratification, consistency in undisturbed and remolded states, moisture and drainage conditions.		P. P.		- OH
	Silts and Clays Liquid limit greater than 50	мн	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts.	Slight to medium	Slow to none	Stignt to medium	Example:  Clayey silt, brown, slightly plastic small		10	ML ML	70 80 90 10	
		Silts Liqu greaté		Organic ctays of medium to high plasticity, organic silts.	Medium to high	None to very slow	Slight to medium	percentage of fine sand, numerous vertical root holes firm and dry in place, loess (ML).		٥	LIQUID LIMI	T HART
		onic Soils	Pt	Peat and other highly organic soils.	1	Albumum daudura	, spongy feel and		┸		- For laboratory classification of	fine-grained soils

characteristics of two groups are designated by combinations of group symbols. For example GW-GC, well-graded gravel-sand mixture with clay binder. (2)All sieve sizes on this chart are U.S. star FIELD IDENTIFICATION PROCEDURES FOR FINE-GRANED SOIL OR FRACTIONS
These procedures are to be performed on the minus No. 40 sieve size particles, approximately 1/64 in. For field classification purposes, screening is not intended, simply remove by nand the coarse particles that interfere with the tests.

Dilotancy (Reaction to shoking)

After removing particles larger than No.40 sieve size, prepare a pot of most soil with a volume of about one-half cubic inch. Add enough the soil with a volume of about one-half cubic inch. Add enough the soil soft but not sticky. The stiff has been point of one hand and shake horizontally, striking vigorously against the other hand several times. A positive reaction consists of the appearance of water on the suface of the path which changes to a livery consistency and becomes glossy. When the sample is squeezed between the fingers, the water and gloss disappear from the surface, the pat stiffers, and finally it crocks or tist disappearance of water during shaking and tist disappearance of water during shaking and tist disappearance during squeezing assist in identifying the character of the fines in a soil.

Very fine clean snnds give the quickest and most distinct reaction whereas a plastic clay has no reaction inorganic sits, such as a typical rock flow, show a moderately quick reaction.

Dry Strength (Crushing characteristics)

Dry Strength (Crushing characteristics)

After removing particles larger than No.40 sieve size, mold o pat of soil to the consistency of putty, adding vater if necessary. Allow the pat to dry completely premium, and are removed to the consistency of putty and the consistency of the character and the pat to the collection of the character of the character of the consistency of the collection of the collect

Toughness (Consistency near plastic limit)

After removing particles larger than the No.4O sieve size, a specimen of soil about one-half inch cube in size, is molded to the consistency of putty. If too dry, water must be added and its disky, the specimes should be spread out in a thirl layer and allowed to lose some moisture by evaporation. Then the specimes is rolled out by hand on a smooth surface or moisture by evaporation. Then the specimes is rolled out by hand on a smooth surface or between the palms into a thread about the manipulation in the moisture content is gradually reduced and the specimen stiffens, finally loses its plasticity, and crumbles when the plastic limit is reached.

After the thread combles, the pieces should be lumped together and a slight kneading action continued unit he map crumbles.

The fougher the thread near the plastic limit and the stiffer the lump when it finally crumbles, the more potent is the callidad clay fraction in the soil. Weakness of the thread of the plastic limit and quick loss of coherence of the lump below the plastic limit indicate either inargers, and in the speciment of the lump below the plastic limit indicate either inargers, and in the control of the plastic limit indicate either inargers, and the plastic limit and appears the course of the line.

Highly organic clays have a very weak and spongy feel at the plastic limit.

Adopted by Corps of Engineers and Bureau of Reclamation, January 1952

THE ABOVE CHART CONSISTS OF COMPLETE DATA AS INCLUDED IN CORPS OF ENGINEERS' UNIFIED SOIL CLASSIFICATION SYSTEM. SOIL CLASSIFICATION NOT APPLICABLE TO THIS PROJECT SHALL BE DISREGARDED.

#### LEGEND

LEGEND FEATURES NOT APPLICABLE TO THIS PROJECT SHALL BE DISREGARDED

- F = FILL
  T = TOP SOIL
  OB = OVERBURDEN(SEE GENERAL NOTE 4)
  SS = SANDSTONE
  SH = SHALE
  SS = SILT STONE
  LS = LIMESTONE
  GN = GRANITE
  CO = COAL
  UC = UNDERCLAY
  (W) = WEATHERED
  GENERAL NOTE 2)
  WEGNITH RED
  GENERAL NOTE 2)
  WEGNITH BADATE
  WEIGHT 18 DATE
   CLASSIFICATION VERIFIELD BY LABORATORY TESTS (SEE GENERAL NOTE 5)

  DC-10 = CORVINC NO A DATE MADE
- DC-10 BORING NO. & DATE MADE
- LT = LIGHT
  DK = DARK
  SL = SLIGHTLY
  MOD = MODERATELY
  MED = MEDIUM
  COMP-COMPACT
  HD = HARD
  TR = TRACE
  F/W = FREE WATER

- W/ = WITH
  GR = GRAIN
  OCC = GRA
- WHEREVER THE METHOD OF EXPLORATION PRECLIDED THE POSSIBILITY OF REDOVERING SAMPLES ABOVE ROCK, SUITABLE FOR EXAMINATION OR TESTS, THE MATERIAL IS DENOTED AS OVERBURDEN, HOWEVER WHERE POSSIBLE FOR THE OVERBURDEN RESIDLE IN THE DRILLING WATER WAS VISUALLY CLASSIFIED IN THE FIELD AND IS SO DENOTED ON THE BORING LOG. CLASSIFICATIONS AND PHYSICAL CHARACTERISTICS OF SOIL AND ROCK AS SHOWN ON THE LOGS WERE DETERMINED IN THE FIELD AND LATER SUPPLEMENTED BY ANALYSIS MADE BY THE DISTRICT GEOLOGIST DETERMINED IN THE FIELD STATEMERG LIMIT TESTS AND THE LABORATORY TECHNICIANS. CLASSIFICATIONS OF MATERIALS VERIFIED BY ATTERBERG LIMIT TESTS ARE DENOTED BY AN ASTERISK. ATTERBERG LIMITS WERE DETERMINED IN ACCORDANCE WITH ASJ.M. D-423-54T AND ASJ.M. D-424-54T.

GENERAL NOTES

WATER ELEVATIONS INDICATED (室) IN CORE (C), FISHTAIL (F), DENISON (U), AND WASH (W) BORINGS MAY HAVE BEEN INFLUENCED BY TRAPPED DRILLING WATER AND SHOULD NOT BE CONSTRUED AS INDICATING THE TRUE GROUND WATER LEVEL.

REFUSAL IS DEFINED AS THE POINT BEYOND WHICH FURTHER PENETRATION WAS IMPOSSIBLE WITH THE EXPLORATION METHOD USED. SEE METHODS OF EXPLORATION THIS SHEET INDICATING MEANS BY WHICH THE BORINGS WERE ADVANCED.

GROUND WATER LEVELS WILL VARY IN ACCORDANCE WITH RAINFALL AND STREAM STAGES, THEREFORE, ACTUAL LEVEL OF GROUND WATER ON ANY DATE OTHER THAN THAT SHOWN ON THE LOGS MUST BE DETERMINED BY SEPARATE OBSERVATIONS. THE OMISSION OF GROUND WATER ELEVATIONS SHALL NOT NECESSARILY BE CONSTRUED AS INDICATING THE ABSENCE OF GROUND WATER AT A PARTICULAR BORING LOCATION.

- A.S.I.M. D-423-341 AND A.S.I.M. D-467-341.

  FIELD BOOKS, FIELD LOGS, LABORATORY LOGS (PRESENTING THE RESULTS OF LABORATORY TESTS FOR SHEAR, CONSOLIDATION, PERMEABILITY, ETC, WHEN PERFORMED) AND EDITED LOGS MAY BE FOR SHEAR, CONSOLIDATION, PERMEABILITY, ETC, WHEN PERFORMED) AND EDITING LOUISVILLE, VIEWED IN THE OFFICE OF THE DISTRICT ENGINEER, US. ARMY ENGINEER DISTRICT, LOUISVILLE, CORPS OF ENGINEERS, 830 WEST BROADWAY, LOUISVILLE, KENTUCKY. FAILURE OF THE CORPS OF ENGINEERS, 830 WEST BROADWAY, LOUISVILLE, KENTUCKY. FAILURE OF THE CONTROL OF THE INFOMMATION REPRESENTED BY THE BORING LOGS, CONTRACTOR TO AVIL HIMSELF OF THE INFOMMATION REPRESENTED BY THE BORING LOGS, INCLUDING LABORATORY TEST RESULTS, SHALL NOT BE GROUNDS FOR A CLAIM THAT THE GOVERNMENT WITHELD INFORMATION ON SUBSURFACE CONDITIONS.
- THE TERM "LOST WATER" AS SHOWN ON THE LOGS INDICATES A QUANITY OF DRILLING WATER LOST IN THE DRILL HOLE IN EXCESS OF 40 GALLONS PER MINUTE.

METHODS OF EXPLORATION NOTE: THE LETTER DESIGNATION USED BEFORE BORING NUMBER INDICATES THE METHOD OF EXPLORATION FOR EXAMPLE: DC= DRIVE SAMPLER AND CORE BORING 

R-011953B

DRIVE SAMPLES WERE TAKEN WITH A 2 100 - 3" QD. X 5" SOLID BARREL SAMPLER USING A 350" HAMMEN WITH AN 18" DRIVE OF UNLESS OTHERWISE INDICATED ON GRAPHIC LOGS.

DESCRIPTION REVISIONS MONICAL & WOLVERTON U.S.ARMY ENGINEER DISTRICT, LOUISVILLE CORPS OF ENGINEERS ENGINEERS-ARCHITECTS LOUISVILLE , KENTUCKY INDIANAPOLIS, INDIANA OHIO RIVER BASIN D.P.M. MONROE RESERVOIR TRACED BY: SALT CREEK, INDIANA ROAD RELOCATION CHECKED BY: SITES 1-384 SOIL CLASSIFICATION CHARTS AND LEGENDS APPROVED: CHIEF ENGINEERING DESIGN APRIL 1963 DISTRICT ENGINEER SCALE: AS SHOWN APPROVED FOR DRAWING NUMBER EFW 143 - 12.4/21

